

Hydrologic Flow DETERMINATION for Hydropower Feasibility Analysis

by

J.R. Filler

C.C. Warnick



Idaho Water and Energy Resources Research Institute

University of Idaho

Moscow, Idaho

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J.R. Filler
Research Associate

and

C.C. Warnick
Professor of Civil Engineering

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Idaho Water and Energy Resources Research Institute
University of Idaho
Moscow, ID 83843

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TABLE OF CONTENTS

	Page
LIST OF FIGURES	iii
LIST OF TABLES	iv
ABSTRACT	v
FORWARD	vi
INTRODUCTION	1
PURPOSE AND OBJECTIVES	1
DESCRIPTION OF HYDROLOGIC AREA	2
METHODOLOGIES FOR ESTIMATION OF FLOW	2
Early Methodologies	4
Heitz, et. al. Methodology	5
Other Methodologies	7
COMPARISON AND ANALYSIS OF METHODS	7
Hawley and McCuen Analysis	7
NEW CONCEPTS	19
CONSEQUENCES OF USING DIFFERENT AVERAGE ANNUAL DISCHARGE ESTIMATES IN PREDICTING FLOW DURATION CURVES FOR HYDROPOWER FEASIBILITY STUDIES	23
CONCLUSIONS AND RECOMMENDATIONS	25
REFERENCES	27
APPENDIX	29

LIST OF FIGURES

Figure		Page
1	Map of Areas Involved in Study and Areas Studied by Hawley and McCuen	3
2	Relationship Between K-Values and Area-Precipitation Product for the Clearwater River, Idaho Method	6
3	Idaho, Montana, and Wyoming Region Outlined by Hawley and McCuen	8
4	Methods for Determining Mean Elevation	20
5	Methods for Determining Characteristic Length of Watersheds	21
6	Flow Durations Based on Two Different Gaged Stream Records for Extrapolation	22
7	Flow Duration Curves Based on the Gaged Record of Pack River at Colburn Utilizing Different Methods of Predicting Mean Annual Discharge	24

LIST OF TABLES

Table		Page
1	Results of Regression Analysis of Relation Between Normal Annual Precipitation and Runoff for Original 211 Stations and for 43 Selected Stations	10
2	Regression Equations with Relative Relief and Relative Steepness as Parameters Influencing Runoff-Rainfall Relations Compared with Regression Equations Using Just Precipitation as a Parameter	12
3	Comparison of Predicted Average Discharge with Actual Average Annual Discharge, Using the Hawley-McCuen Equation for Predicting	14
4	Comparison of Runoff Coefficeints Calculated from Hawley-McCuen Studies with Coefficients Calculated from Heitz Studies	15
5	Results of Applying Hawley-McCuen Equation to Ungaged Watersheds in Northern Idaho	16
6	Water Yield at Sample Stations Using the McDonald Study	17
7	Water Yield at Sample Stations Using the Rosa Study	17
8	Average Annual Runoff Using the Kjelstrom Study	18

ABSTRACT

This report presents comparisons of different methods of estimating average annual discharge of ungaged streams. The primary purpose of the estimates being for making hydropower feasibility investigations that utilize flow duration curves. The published methods of McDonald (1948), Rosa (1968), Heitz (1980, 1981) and Hawley-McCuen (1982) were used on selected streams in Idaho to make the comparisons. The methods have been developed over a considerable period of time and have used different periods of record but all have used normal annual precipitation estimates as the basis for prediction. Reasonable agreement was obtained in the comparisons but inconsistencies do appear on some drainages.

Several additional approaches to improving the estimates and hopefully correcting the inconsistencies have been postulated and techniques for study recommended.

FOREWARD

The Idaho Water and Energy Resources Research Institute has provided administrative coordination for this study and organized the team in the Department of Civil Engineering at the University of Idaho that conducted the research. It is the Institute policy to make available the results of significant research related to the water and energy resources within Idaho and for possible application in a national and international realm. The Institute neither endorses nor rejects the findings of the authors. In this study a strong effort has been made to utilize earlier findings of research and to compare different hydrologic techniques in the field of hydropower engineering and to determine limitations that must be accounted for in using data drawn from hydrologic measurements that are frequently very limited in geographic coverage and often of limited historical length. The Institute does encourage careful consideration of techniques recently developed for extending the usefulness of limited hydrologic measurements.

INTRODUCTION

An earlier report entitled, "Assessment of the Usefulness of Hydrologic Data for Hydropower Feasibility Analysis" defined some general limits for the extrapolation of flow data for input in the computation of hydropower design capacity. That earlier study treated specifically the use of flow duration curves and utilized techniques of using parametric duration curve developed by Heitz (1981). An important parameter to make effective use of parametric flow duration curves is the determination of an accurate estimate of the average flow at an ungaged stream site. A recent paper by Hawley and McCuen (1982) has presented regression equations for determining average flow based on measurements of parameters such as precipitation, mean elevation, and percent cover density. The earlier work of the writers of this report has relied on extrapolation from existing streamgage records and qualitative evaluation along with qualitative judgments to temper runoff coefficients on ungaged watersheds such that they are compatible with the runoff coefficients at existing gaged watersheds.

PURPOSE AND OBJECTIVES

The purpose of this supplemental research was to develop a better method or methods for determining average flow at ungaged streamflow sites where hydropower potential exists. The specific objectives were to:

1. Study Hawley-McCuens approach and compare their approach with data developed in earlier research by Heitz and others.
2. Study ways of improving the accuracy of estimating the average flow of ungaged sites in relation to flow duration expressions.

DESCRIPTION OF HYDROLOGIC AREA

The area of principal interest for this research was the drainages in the Pacific Northwest wherein there is considerable topographic relief and a definite orographic effect on storms moving into the continental land masses. This region experiences considerable variation in the normal annual precipitation and the geology and topography make it difficult to generalize runoff characteristics of the streams. The work of Hawley and McCuen covered a major section of the western United States and subdivided the area into five regions as shown in Figure 1. The earlier work of this project concentrated on two major stream drainages in Idaho, the Clearwater River basin and the Salmon River basin. These basins are delineated in a general way on the generalized map adopted for the study by Hawley and McCuen (Figure 1). This study has concentrated most of the research on those basins mentioned but has made comparisons on a sampling basis on other basins in Idaho to compare results obtained in using the regression equations of Hawley and McCuen with earlier estimates of runoff characteristics as developed in the studies by Heitz, et. al (1980) and Warnick, et. al (1981). Particular samplings were made of a further comparison with some unpublished work of Johnson (1982) in the upper portions of Snake River drainage in Idaho.

METHODOLOGIES FOR ESTIMATION OF FLOW

Much research has been done on relating yield of storm runoff from the input parameter of precipitation. Most of the work has been done on short-term precipitation events and is characterized by such studies as those of Linsley and Ackerman (1942), Kohler and Linsley (1951) and the U.S. Soil Conservation Service (1955). Longer-term periods of

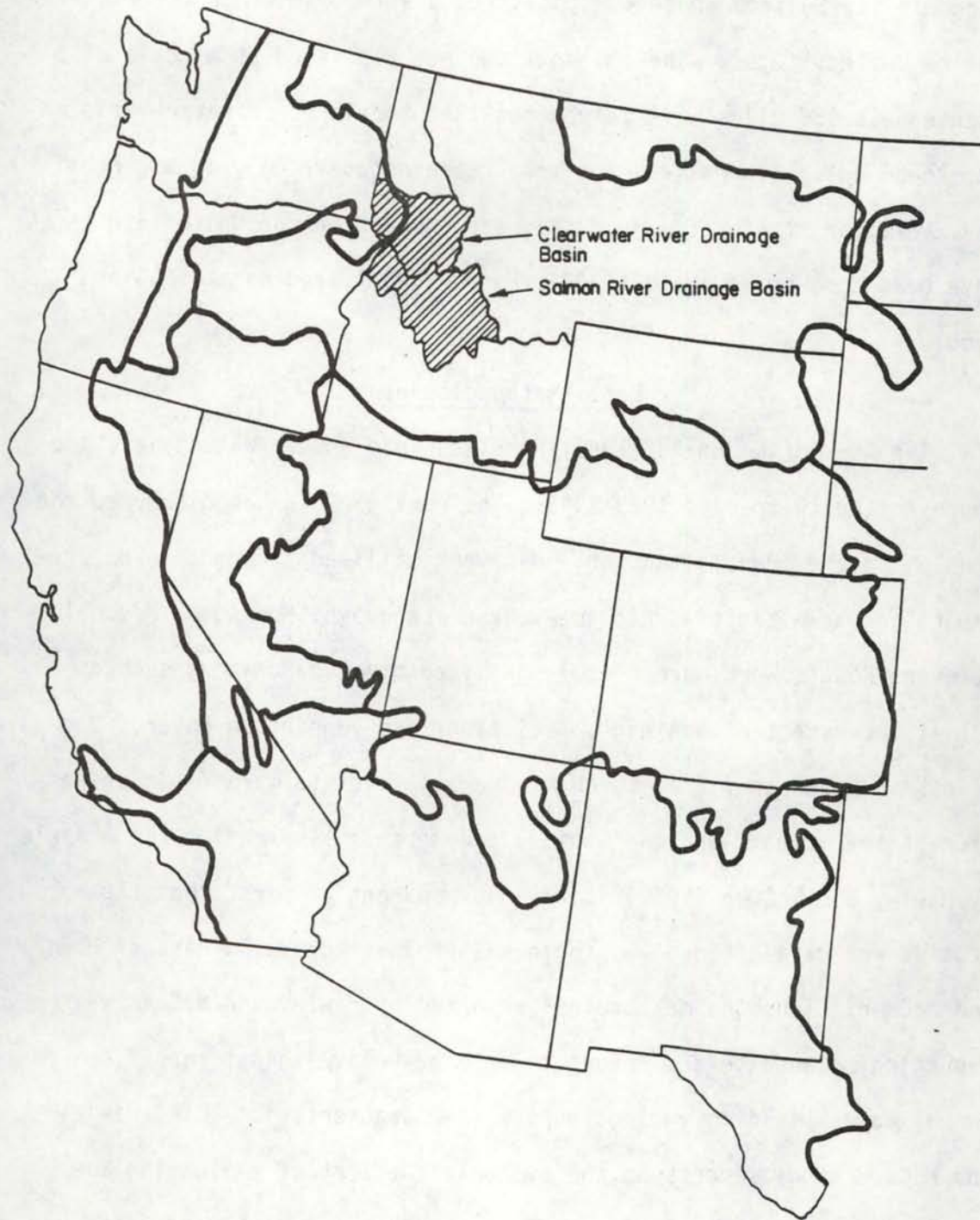


Figure 1. Map of Areas Involved in Study and Areas Studied by Hawley and McCuen.

annual runoff resulting from annual precipitation has been studied by Langbein (1949) from which was published a water yield map covering the entire United States. The original map was published at a scale of 1 inch equals 150 miles. It is obvious that the lines of water yield had to be quite generalized and were representative only in a general way. Similar studies for just the western part of the United States have been done by McDonald (1948), Munns (1952), and Rosa (1956 and 1968).

Early Methodologies

The work of McDonald (1948) resulted in a annual water yield map representing the period 1929-1945. The work of Rosa has pioneered the problem in the Idaho situation. His work utilized information on precipitation as a basis as did the work of Langbein, Munns and McDonald. However, Rosa's work more closely analyzed other parameters such as elevation, aspect of drainage, soil types and vegetative cover. Much of his work was confirmed through numerous trips to make field inspection of the streams and the various parameters. In reading the article by Hawley and McCuen (1982) it was not apparent at first that the studies and data of Rosa was the basis of the regression data of Hawley and McCuen. Thus the methodology reported by Hawley and McCuen was reporting on an extended effort to relate average annual runoff or annual water yield to various watershed characteristics. The Hawley and McCuen study reports on the systematic effort of evaluating the significance of 17 predictive variables. These variables or watershed parameters were (1) drainage area, (2) percent bare ground, (3) percent cover density, (4) mean elevation, (5) minimum elevation, (6) elevation at temperature station, (7) maximum elevation, (8) latitude, (9) longitude, (10) mean annual precipitation, (11) minimum average annual

precipitation, (12) maximum average annual precipitation, (12) 60-minute, 25-year precipitation, (14) soil group, (15) mean annual temperature, (16) mean annual temperature at station, (17) percent vegetation and litter, and the dependent variable, water yield. The conclusion showed that the most significantly important parameters were mean annual precipitation, mean elevation, and percent cover density. In fact, the lone variable of precipitation tended to give just as good of results for a predictive evaluation of water yield as did including the other parameters. This work was done on mostly small gaged watersheds and appeared to have used no common historical time period.

Heitz, et. al. Methodology

The work of Heitz, Warnick, and Emmert (1979) approached the problem by attempting to use relations of average annual runoff or flow as related to input mean annual precipitation utilizing comparisons between gaged streams in a given hydrologically similar area. A certain amount of subjective engineering judgement was used to choose the appropriate runoff coefficients. Figure 2 taken from Emmert's (1979) thesis shows how some of the projections were made. Runoff coefficients were developed for all the reaches of rivers studied under an inventory of potential hydropower energy in the streams of Idaho; Heitz, et. al. 1980. This information is best catalogued in a report by Warnick, Heitz, Kirkland and Burke (1981). The subjective technique of determining runoff coefficients was guided by the fact that at gaged sites the accumulated effect had to equal the ratio of the measured average annual runoff to the calculated mean annual precipitation as determined from long-time isohyetal maps of normal annual precipitation.

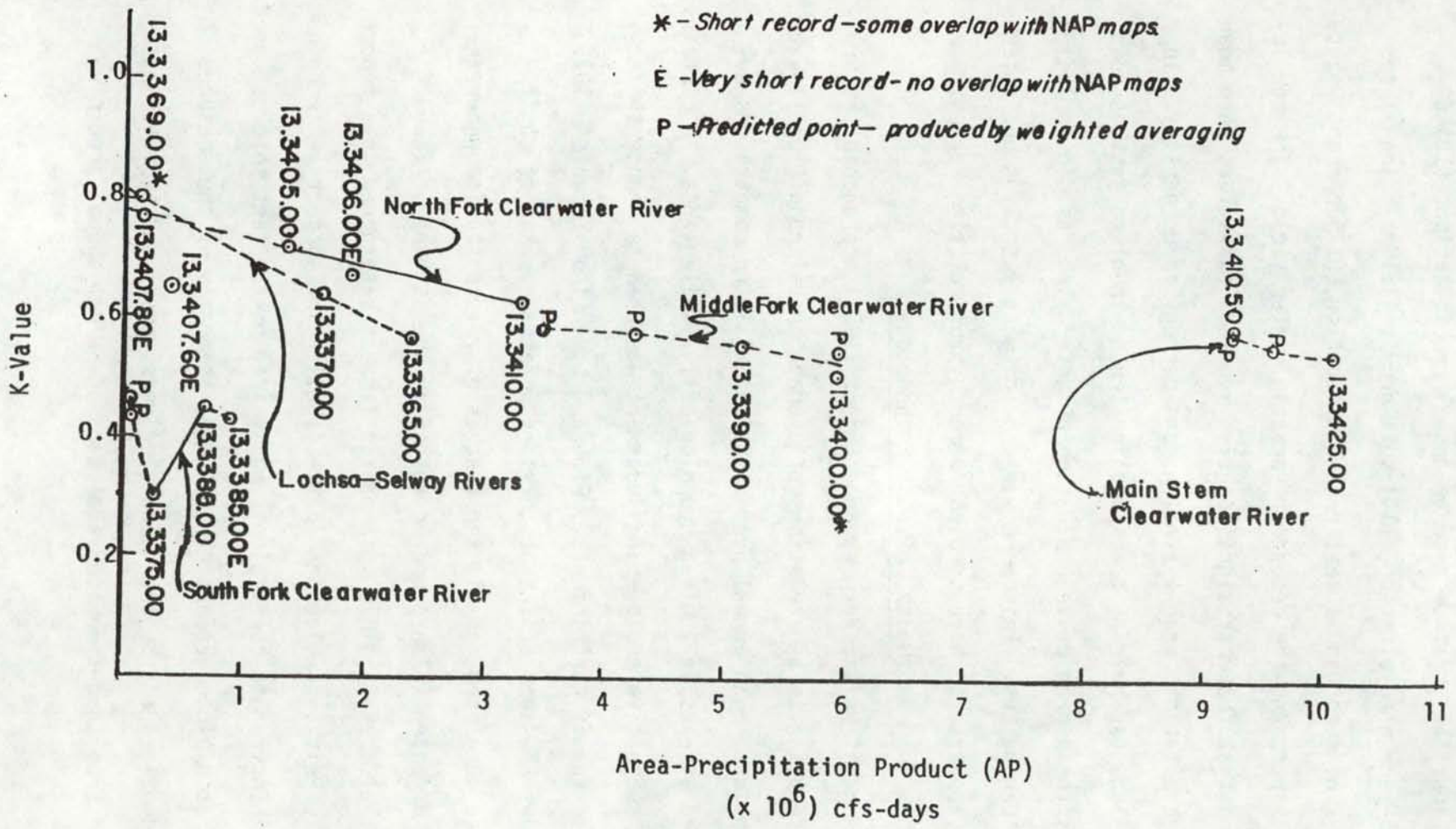


Figure 2. Relationship Between K-Values and Area-Precipitation Product for the Clearwater River, Idaho Method. Source: Emmert (1979)

Other Methodologies

In the research work of Gary Johnson (1982) there is reported another approach to determining average annual runoff. This is the unpublished work of Kjelstrom of the Boise Office of the U.S. Geological Survey and is for regions of the Upper Snake River drainage in Idaho. The particular equation for estimating average annual runoff was:

$$Q = 0.000903 A^{0.9} P^{1.83} (F)^{0.29} \quad \text{Eq. (1)}$$

where Q = average annual runoff in CFS

A = watershed area in square miles

P = mean or normal annual precipitation in inches

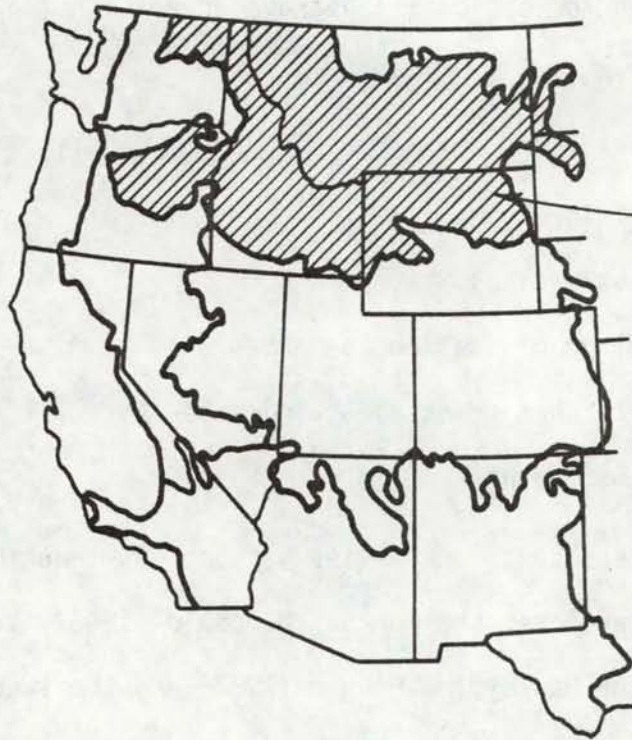
F = percentage of the watershed forested as indicated by
1:250,000 scale U.S.G.S. maps

The work of Rosa (1968) represents an earlier methodology than the work of Hawley and McCuen. Therefore, the work of McDonald (1948), the work of Rosa (1968), the work of Hawley and McCuen (1982) and the studies of Warnick, et. al. (1981) all represent different approaches. The research results reported here then are an effort to compare these various methodologies on some hydrologic situations in Idaho that have particular importance in feasibility studies of hydropower.

COMPARISON AND ANALYSIS OF METHODS

Hawley and McCuen Analysis

Analyses of the work of Hawley and McCuen was conducted on only the region they called Idaho-Montana-Wyoming and really covering the area that is cross-hatched in Figure 3. This actually involved 211 watersheds that had measured data for which analysis was possible. An



Idaho, Montana, and
Wyoming Region Outlined
by Hawley and McCuen.

Figure 3. Idaho, Montana, and Wyoming Region
Outlined by Hawley and McCuen.

appraisal of those sites revealed several gaged watersheds that the writers question as having good records applicable to this study. One site appears to have been recorded twice. A decision was made to utilize only 43 watersheds that were considered representative of good stations involving Idaho-type conditions. With the original data from the Hawley and McCuen report a regression study was made to see whether the selected group of 43 watersheds revealed different regression relations than the original 211 watersheds and if there was evidence that a better correlation might be obtained. Results of this analysis are presented in Table 1. The base data for these results are presented in the Appendix.

An alternative method of utilizing the same basic factors as the Hawley and McCuen data was also considered. This involved introducing parameters that were easily calculated from the basic data that were identified in the Hawley and McCuen information which were considered to have an influence on the runoff-precipitation relationship. The first new parameter was a relative relief term, ΔE , the difference between maximum elevation and minimum elevation of the basin. This was considered because with relatively large differences in elevation within a watershed it was considered that one might expect increased precipitation and more runoff coming off the watershed in a shorter period of time thus, allowing less loss of water from the annual precipitation input. This parameter was used in two regression analysis with both the original 211 sets of data and with the selected 43 watersheds data. One used normal annual precipitation, P_a , ΔE , and cover density, C , as the parameters to relate to average annual runoff. The other analysis used normal annual precipitation, P_a , mean elevation, E_a , relative relief, ΔE , and cover density, C .

Table 1. Results of Regression Analysis of Relation Between Normal Annual Precipitation and Runoff for Original 211 Stations and for 43 Selected Stations

<u>No. of Sites</u>	<u>Prediction Equation</u>	<u>R</u>	<u>R²</u>
211	$Y = -12.1721 + 0.91756 P_a$	0.973	0.947
43	$Y = -12.801182 + 0.908355 P_a$	0.939	0.882
211	$Y = 2.10 \times 10^{-5} P_a^{2.45458} E_a^{0.49269} C^{0.16526}$	0.926	0.857
43	$Y = 1.060183 \times 10^{-3} P_a^{2.263636} E_a^{0.093715} C^{0.222769}$	0.934	0.872

Y = runoff or water yield in inches
P_a = normal annual precipitation in inches
E_a = mean elevation of basin ft. msl.
C = percent cover density, %

A further inquiry was to consider two relative steepness terms, S_R , and S'_R . This was defined by two different methods as shown in the two equations below:

$$S_R = \frac{E_{\max} - E_{\min}}{\sqrt{A}} \quad \text{Eq. (2)}$$

where S_R = relative steepness

E_{\max} = maximum elevation in ft

E_{\min} = minimum elevation in ft

A = drainage area in mi^2

and

$$S'_R = \frac{E_{\max} - E_{\min}}{A} \text{ or } \frac{S_R}{\sqrt{A}} S_R \quad \text{Eq. (3)}$$

Regression analyses were made with these terms as additional parameters to the normal annual precipitation and cover density terms related to average annual runoff. The resulting equations for the regression relations of the new parameters and the correlation coefficients are shown in Table 2. Very little change in the correlation coefficients was obtained.

A comparison was then made between the historical mean annual discharge and the predicted mean annual discharge as computed from the empirical regression equation developed by Hawley and McCuen (1982), namely:

$$Y = -12.1721 + 0.91756 P_a, \quad \text{Eq. (4)}$$

where Y = runoff or water yield in inches, and

P_a = mean annual precipitation in inches.

Table 2. Regression Equations with Relative Relief and Relative Steepness as Parameters Influencing Runoff-Rainfall Relations Compared with Regression Equations Using Just Precipitation as a Parameter

No. of Sites	Prediction Equation	R	R ²
211	$Y = 2.215981 \times 10^{-4} P_a^{2.522908} \Delta E^{0.208204} C^{0.182155}$	0.921	0.848
43	$Y = 8.962455 \times 10^{-4} P_a^{2.271726} \Delta E^{0.111674} C^{0.240633}$	0.936	0.875
211	$Y = 1.147326 \times 10^{-5} P_a^{2.437664} E_a^{0.43686} \Delta E^{0.133735} C^{0.183498}$	0.927	0.859
43	$Y = 5.061742 \times 10^{-4} P_a^{2.256129} E_a^{0.071116} \Delta E^{0.108424} C^{0.249435}$	0.936	0.876
211	$Y = 5.318792 \times 10^{-4} P_a^{2.370810} S_R^{0.229934} C^{0.147793}$	0.926	0.857
43	$Y = 1.778558 \times 10^{-3} P_a^{2.293415} S_R^{0.037890} C^{0.209949}$	0.934	0.817
211	$Y = 1.264297 \times 10^{-3} P_a^{2.431168} S_R^{0.093227} C^{0.138412}$	0.922	0.850
43	$Y = 2.446685 \times 10^{-3} P_a^{2.280049} S_R^{0.00803996} C^{0.212218}$	0.933	0.871
<p>Note: $S_R = E / \sqrt{A}$ $S_R' = \frac{S_R}{\sqrt{A}}$ A = drainage area in Mi²</p>			
211	$Y = 1.702990 \times 10^{-3} P_a^{2.6280989}$	0.914	0.835
43	$Y = 3.159716 \times 10^{-3} P_a^{2.434996}$	0.931	0.867

This was done for groups of stations that have gaged streamflows of a significant length and appear to be of importance in hydroelectric studies. The precipitation input values for each watershed were those obtained from the studies by Warnick, et. a. (1981). None of these stations used in the comparisons were included in the development of Hawley and McCuens's regression equation, (Equation 3). The results of the comparison are shown in Table 3.

It was possible to find a few measured streamflow stations that were common in the studies of Hawley and McCuen (1982) and Heitz, et. al. (1980) wherein data were available to compare the runoff coefficients determined with the aid of the two regression equations of Hawley and McCuen and those determined in the studies by Heitz. This comparison is shown in Table 4. Note, this has been done with different values of precipitation (normal annual precipitation) and with different methodologies for getting the runoff coefficient.

In the final effort using the regression equations of Hawley and McCuen an attempt was made to apply the approach to ungaged streams and to compare the results with results obtained by using the methodology developed in the hydropower inventory study of Heitz et. al. (1980). Results for these are shown in Table 5.

In addition to the comparison of values predicted by the Hawley and McCuen equation, water yield or average annual discharge values were predicted from other sources and compared with gaged-record data. Results of the comparison are shown in Tables 6, 7 and 8. It should be noted that in Table 8 the comparison was made with the mean annual discharge for water year 1963 for the three gages shown. Water year 1963 was selected as it represented fairly closely the long-term average condition for other gaged sites in Idaho.

Table 3. Comparison of Predicted Average Discharge With Actual Average Annual Discharge, Using the Hawley-McCuen Equation for Prediction
 $(Y = -12.1721 + 0.91756 P_{NAP})$

<u>Gage No.</u>	<u>Gage Location</u>	<u>Area</u> (mi ²)	<u>Reach No.</u>	<u>P_{NAP}</u> (in)	<u>Y</u> (m)	<u>\bar{Q}</u> (cfs)	<u>\bar{Q}</u> 1930-57 (cfs)
<u>Salmon River Drainage</u>							
13.2950.00	Valley Creek-Stanley	147	088R0080	27.5	13.06	141.11	199.37
13.2955.00	Salmon River-Stanley	535	000R0082 & 088R0080	34.78*	19.74	778.05	669.88
13.2965.00	Salmon River-Clayton	802	000R0078 & 085R0002	33.14*	18.24	1077.43	992.57
13.2985.00	Salmon River-Challis	1800	000R0062	31.2	16.46	2182.13	1464.06
13.3025.00	Salmon River-Salmon	3760	000R0050	27.9	13.43	3719.50	1902.94
13.3085.00	Middle Fork-Cape Horn	138	030R0056	31.9	17.10	173.83	239.30
13.3090.00	Bear Valley Creek-Cape Horn	180	030R0052	35.0	19.94	264.45	294.31
13.3105.00	South Fork-Knox	95	020S0015	41.4	25.81	180.67	146.57
13.3130.00	Johnson Creek-Yellow Pine	213	020R0020	36.2	21.04	330.21	343.56
13.3170.00	Salmon River-Whitebird	13,550	000R0008	28.9	14.35	14,319.97	10,762.84
<u>Clearwater River Drainage</u>							
13.3365.00	Selway River-Lowell	1910	020R0105	46.2	30.22	4252.13	3641.86
13.3370.00	Lochsa River-Lowell	1180	020R0010	50.6	34.26	2977.93	2752.61
13.3390.00	Clearwater River-Kamiah	4850	000R0028	39.9	24.44	8731.88	7755.90
13.3410.00	North Fork-Ahsahka	2440	Dworshak	50.2	33.89	6091.78	5533.21
13.3425.00	Clearwater River-Spalding	9570	002S0002 & 000R0010	41.48*	25.61	18,057.74	14,737.09
<u>Other Gages in Idaho</u>							
12.3055.00	Boulder Creek-Leonia	53	000R0003	42.6	26.92	105.09	116.12
12.4145.00	St. Joe River-Calder	1030	000R0008	50.6	34.26	2599.38	2295.81
12.4150.00	St. Maries River-Lotus	437	010R0004	39.6	24.16	779.91	517.34
13.1850.00	Boise River-Twin Springs	830	000R0013	40.9	25.36	1150.43	1169.56

*Weighted average for two reaches.

Table 4. Comparison of Runoff Coefficients Calculated from Hawley-McCuen Studies with Coefficients Calculated from Heitz Studies

Gage No.	Site Name	P_a	P_{NAP}	Y_{HM}	K_{HM}	$Y=f(P_a)$	$K=f(P_a)$	$Y=f(P_a, E_a, C)$	$K=f(P_a, E_a, C)$	Y_{HZ}	K_{HZ}
		(in)	(in)	(in)	(in/in)	(in)	(in/in)	(in)	(in/in)	(in)	(in/in)
13.3369.00	Fish Creek-Lowell R.S.	50.00	45.89	38.70	0.77	33.71	0.67	37.30	0.75	36.7	0.80
13.3165.00	Little Salmon-Riggins	30.00	31.12	20.00	0.67	15.35	0.51	11.64	0.39	18.4	0.59
13.3414.00	E.F. Potlatch-Bovill	40.00	42.97	20.30	0.51	24.53	0.61	20.21	0.51	20.2	0.47
13.2610.00	Little Weiser-Indian Valley	28.00	34.85	17.00	0.61	13.52	0.48	10.33	0.37	17.6	0.48
13.3115.00	E.F.S.F. Salmon-Stibnite	35.00	35.56	16.10	0.46	19.94	0.57	20.24	0.58	20.3	0.57
13.3125.00	Johnson Creek-Landmark	45.00	40.83	22.50	0.50	29.12	0.65	36.67	0.81	20.4	0.50
13.2005.00	Robie Creek-Arrowrock	25.00	22.32	7.10	0.28	10.77	0.43	6.58	0.26	6.9	0.31

P_a = Precipitation Data from Hawley-McCuen Studies (in)

P_{NAP} = Precipitation Data from Heitz Studies (in)

Y_{HM} = Runoff or Water Yield Data from Hawley-McCuen Studies (in)

K_{HM} = Calculated Runoff Coefficient, Y_{HM}/P_a (in/in)

$Y=f(P_a)$ = Predicted Runoff or Water Yield Using $Y=12.1721 + 0.91756 P_a$

$K=f(P_a)$ = Runoff Coefficient Calculated Using Predicted Runoff, $K=[Y=f(P_a)]/P_a$

$Y=f(P_a, E_a, C)$ = Predicted Runoff Using $Y=2.1 \times 10^{-5} P_a^{2.45458} E_a^{0.49269} C^{0.16526}$

$K=f(P_a)$ = Runoff Coefficient Calculated Using Predicted Runoff, $K=[Y=f(P_a, E_a, C)]/P_a$

Y_{HZ} = Runoff Calculated Using Precipitation Data from Heitz Studies

K_{HZ} = Runoff Coefficient from Heitz Studies

Table 5. Results of Applying Hawley-McCuen Equation to Ungaged Watersheds in Northern Idaho.

Site	\bar{P} (in)	$Y=f(\bar{P})$ (in)	$\bar{Q}=f(\bar{P})$ (cfs)	\bar{Q}_{EST} (cfs)
Elk Creek at Elk Creek Falls	47.10	31.04	108.59	94.33
Hell Roaring Creek above Mouth	46.10	30.13	24.95	29.80
Bedrock Creek above Mouth	22.55	8.59	21.20	22.44

16

\bar{P} = Predicted Normal Annual Precipitation by Warnick-Filler (in)

$$Y=f(\bar{P}) = -12.1721 + 0.91756 \bar{P}$$

Q = Average Annual Discharge (CFS)

$$\bar{Q} = f(\bar{P}) = C \times A \times Y = f(\bar{P}) = C \times A \times [-12.1721 + 0.91756\bar{P}]$$

Where A = Area in mi^2 and C = conversion factor 0.07367 cfs-year/in- mi^2

\bar{Q}_{EST} = Average Annual Discharge as predicted by Warnick-Filler (CFS)

Table 6. Water Yield at Sample Stations Using
The McDonald Study

Gage No.	Site Name	Y_{MC}	\bar{Q}_{MC}	\bar{Q}_{1963}	$\bar{Q}_{1930-57}$
		(in)	(cfs)	(cfs)	(cfs)
12.3923.00	Pack River - Colburn	17.50	159.86	302	---
13.1170.00	Birch Creek - Reno	1.00	23.57	70	---
13.3165.00	Little Salmon - Riggins	13.00	551.64	876	---
13.3370.00	Lochsa River - Lowell	30.00	2607.92	---	2752.61

Y_{MC} = Runoff or Water Yield Calculated Using McDonald Method (in)

Table 7. Water Yield at Sample Stations Using
The Rosa Study

Gage No.	Site Name	Y_R	\bar{Q}_R	\bar{Q}_{1963}	$\bar{Q}_{1930-57}$
		(in)	(cfs)	(cfs)	(cfs)
12.3923.00	Pack River - Colburn	28.74	262.54	302	---
13.1170.00	Birch Creek - Reno	3.64	85.81	70	---
13.3165.00	Little Salmon - Riggins	15.16	643.30	876	---
13.3370.00	Lochsa River - Lowell	30.18	2623.57	---	2752.61

Y_R = Runoff or Water Yield Calculated Using the Rosa Study (in)

Table 8. Average Annual Runoff Using
the Kjelstrom Study

Gage No.	Site Name	P_a	P_{NAP}	Q_j	\bar{Q}_{1963}
		(in)	(in)	(cfs)	(cfs)
13.0757.00	South Fork Pocatello Cr.	14.50	---	0.83	0.219
13.1170.00	Birch Creek - Reno	---	28.90	187.63	70.00
13.2070.00	Spring Valley Creek	17.00	---	4.02	1.08

Q_j = Average annual runoff predicted using Kjelstrom method (cfs)

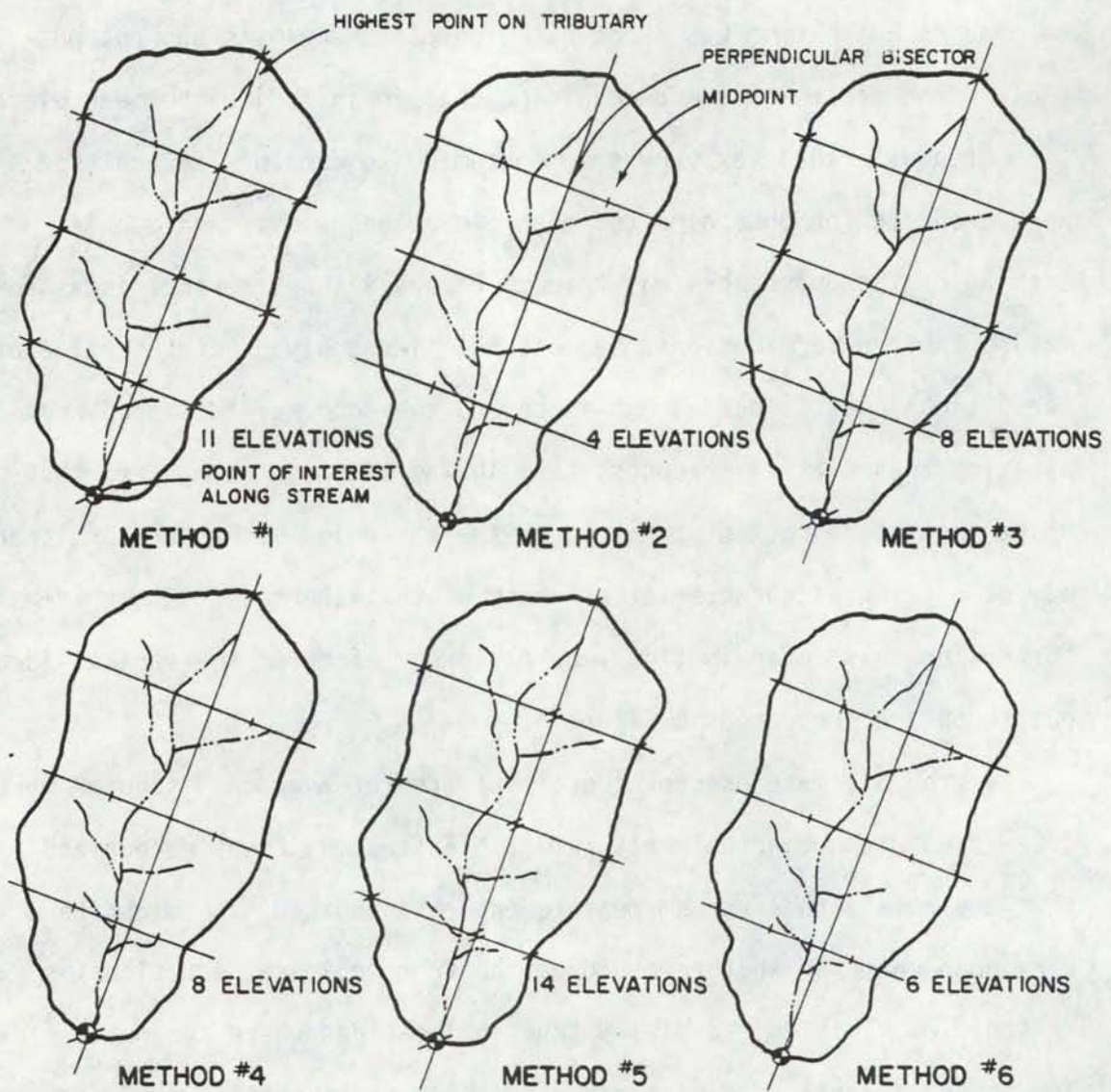
Note:

\bar{Q}_{1963} representative of long-term average conditions

NEW CONCEPTS

During the progress of this research several valuable ideas came to mind on which time was spent but a complete analysis was not possible. The first was the recognition that in including the mean elevation of a watershed was very time consuming to obtain. Several ideas were proposed for obtaining the mean elevation quickly and a brief listing of the approaches was tried. Figure 4 shows several ideas for making these determinations. A more detailed analyses of the value of these techniques is needed but is beyond the scope of this project. In assessing the relative steepness term it was likewise recognized that using the square root of the area of the watershed was a very arbitrary way of getting a characteristic length of the watershed. Other ideas for making this determination were explored. Some of the various ideas put forth are presented in Figure 5.

As the ultimate uses of these estimates of average discharge were often used without actual measurements in streams it was recognized that a single actual stream measurement at a ungaged site might be a very good check on whether a projection from empirical equations is reasonable. This is especially true in locations where the water flowing in the channel at a gage may not represent the entire flow discharging out of a particular basin. Estimates of the duration curves for an ungaged stream location based on flow records of two nearby gaged are shown in Figure 6. This prompted the idea that it would be good to know what single date in the year represents a date when a single measurement will be most consistent in representing a nearly constant relation to the average annual flow and a flow from which to project flow duration curve estimates. This idea needs further research.

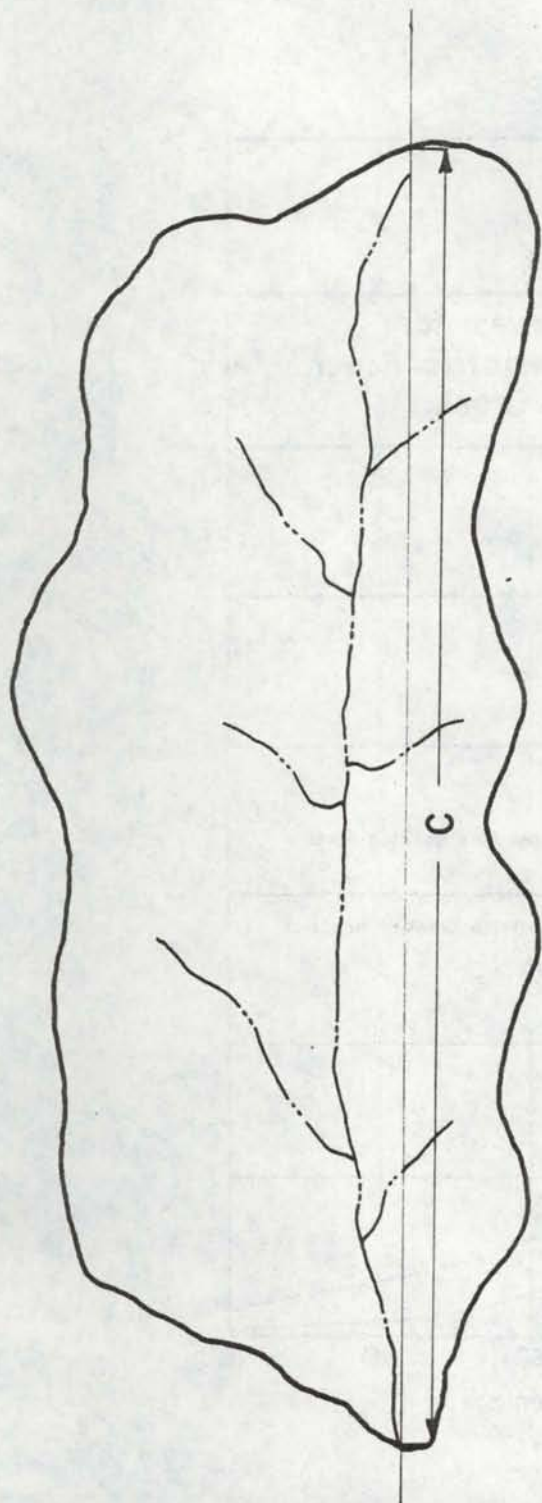


METHOD #7

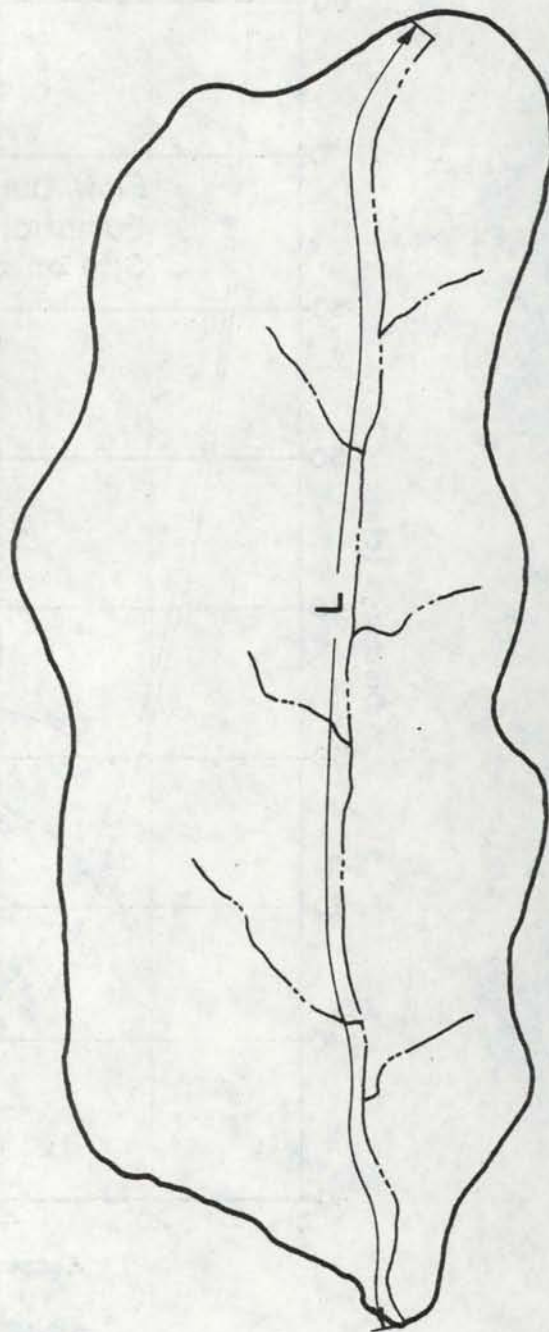
$$\frac{(\text{MAXIMUM ELEVATION} - \text{MINIMUM ELEVATION}) \times 0.34 + \text{MINIMUM ELEVATION}}{1}$$

NOTE: TICK MARKS DENOTE POINTS OF ELEVATION.

Figure 4. Methods for Determining Mean Elevation.



C = CHORD LENGTH THROUGH
LONGEST TRIBUTARY



L = LENGTH OF LONGEST
TRIBUTARY

Figure 5. Methods for Determining
Characteristic Length of Watersheds.

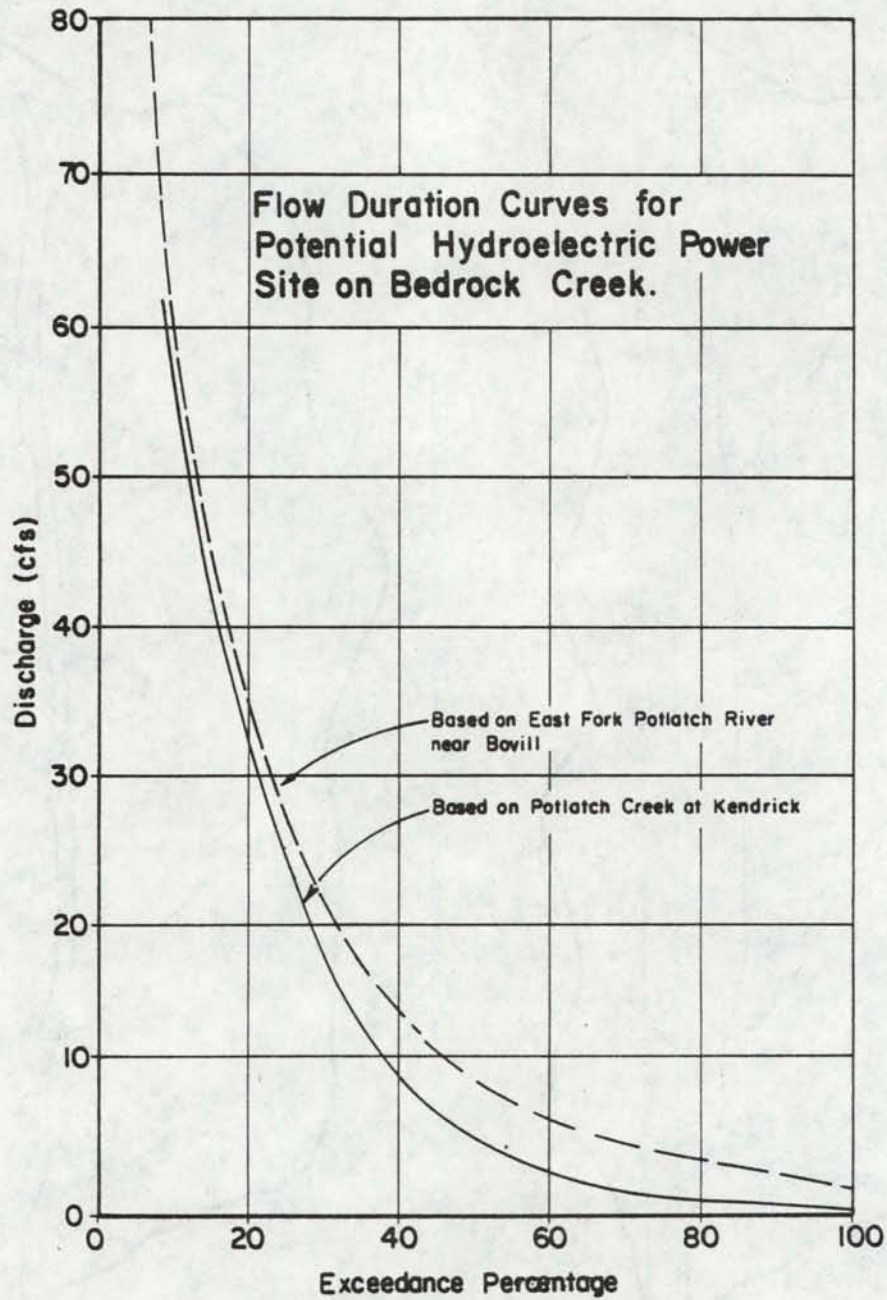


Figure 6. Flow Durations Based on Two Different Gaged Stream Records for Extrapolation.

CONSEQUENCES OF USING DIFFERENT AVERAGE ANNUAL DISCHARGE
ESTIMATES IN PREDICTING FLOW DURATION CURVES FOR HYDROPOWER
FEASIBILITY STUDIES

Because the original research of the project was centered on determining better methods on extrapolating flow duration curves for hydropower studies, it was considered important to project from this research the consequences of using the different methods of estimating average annual discharge and what the actual impact is on an estimate of the flow duration curve for a particular situation. In Figure 7, the dimensionless flow duration curve for the Pack River at Colburn was used to develop the flow duration curve of the Hell Roaring Creek using the several different methods for estimating average annual runoff from an ungaged basin.

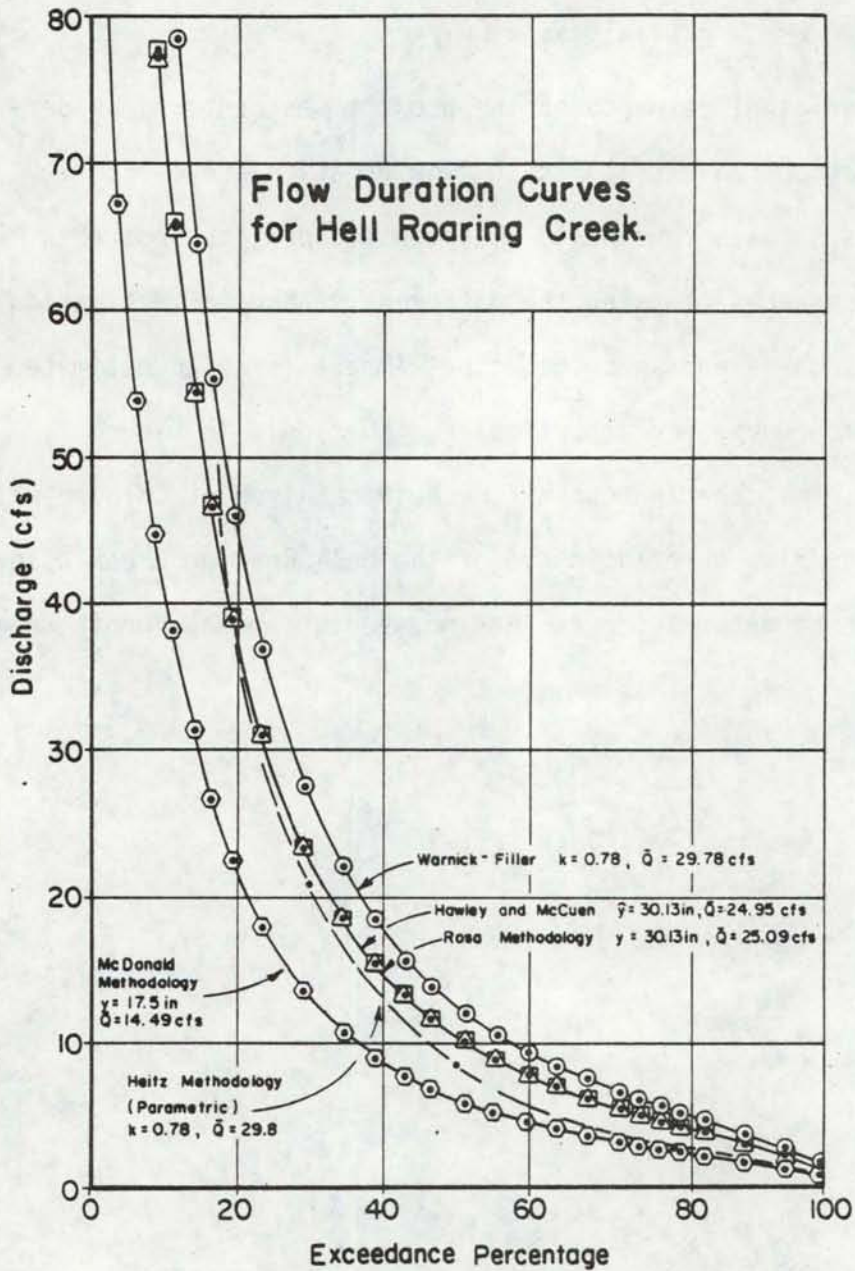


Figure 7. Flow Duration Curves Based on the Gaged Record of Pack River At Colburn Utilizing Different Methods of Predicting Mean Annual Discharge.

CONCLUSIONS AND RECOMMENDATIONS

This research has shown that the single parameter of normal annual precipitation offers the simplest factor to use in estimating average annual discharge from a watershed. Brief study of the work of Langbein, McDonald, Rosa, and Hawley and McCuen has provided methods of determining average annual discharge or annual water yield for Idaho watersheds. However, the earlier studies of Langbein and McDonald were so generalized that the accuracy on small watersheds is not very good. Comparisons utilizing the studies of Rosa, Hawley and McCuen, and of Heitz show reasonable consistency.

These various water yield maps or equations presented by Langbein, McDonald, Rosa, and Hawley and McCuen do represent possible data and methods for making determinations of average annual discharge. The fact that these studies have been developed over a considerable period of time (1948 to 1982) and are based on hydrologic record periods that are of differing duration make it evident that a high degree of consistency cannot be expected. The brief check utilizing an equation suggested by Kjelstrom did not give very satisfactory predictions at the three sample gaged watersheds.

Study of Hawley-McCuen water yield regression equations has revealed that a method of obtaining a plant cover density term is not readily available throughout the state. Needed is a method/s that can be quickly done with consistent and reproducible results.

The comparative result of using different methods of estimating average annual runoff to make predictions of flow duration curves for an ungaged stream, Hell Roaring Creek, shows that the methods do give

different results as shown in Figure 7. The results based on the subjective estimates made by the writers give the highest values. The use of the Heitz, Hawley-McCuen and Rosa approaches show rather close agreement. The use of McDonald's estimates of average annual runoff are apparently too conservative.

The study of predictions of a flow duration curve for a stream based on two nearby gages wherein different extrapolations were obtained as shown in Figure 6 makes it important that measurements of flow be made to attempt to verify the correct flow duration characteristics. A recommendation is made that more research be conducted to determine when measurements should be made that will provide the best data for projecting what the flow duration curve is at the site being investigated.

Because some improvement was observed in the R^2 values for regression equations using additional parameters than just precipitation, it is recommended that additional research be conducted with multiparameters such as relative relief and relative steepness in an attempt to get the best method for making estimates of average annual discharge at ungaged sites.

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APPENDIX

Basic Data for 43 Selected Watersheds as Extracted
from the Original Information of the Hawley-McCuen Study

43 Stations as Selected by J.R. Filler

SELECTED HAWLEY AND MCCUEN SITES

S O I B T S E	L O C A T I O N	M E A N	M E A N	S C O R E	C O V E R A G E	M I N I M U M	B V L A R G E L I T H	M A X I M U M	L O N G I T H	P E R C E N T	T E M P E R A T U R E	P R E C I P I T A T I O N	Y E A R	P E R S O N S							
															V	P	P	N	A	V	P
1	GROUSE	ARROWRK	4450	44.8	20.5	2.0	15	8.00	3400	18	40	25	43.35	5400	22	115.55	3239	49.0	0.7	4.65	0.5812
2	COTN G	BOISE	4200	46.2	16.0	2.0	20	16.00	2790	15	35	30	43.37	5500	20	116.11	2841	51.0	0.7	2.50	0.1563
3	S FORK	POCATELLO	5770	42.4	14.5	3.0	38	4.30	4835	14	29	59	42.53	6790	15	112.23	4444	47.0	1.0	0.65	0.1512
4	SALMON	OBSDIAN	8110	31.2	30.5	1.5	40	94.70	6950	16	18	57	43.58	10175	45	114.48	6870	35.4	0.8	10.40	0.1098
5	ELK	IRWIN	7800	34.6	35.0	1.5	40	7.00	5800	20	18	57	43.24	9770	50	111.01	5397	43.0	1.1	15.20	2.1714
6	PEBBLE	MCCAM	7500	36.5	26.5	1.5	47	27.50	6000	18	14	75	42.50	9271	35	112.10	4774	46.0	1.0	14.00	0.5091
7	ROCK	HOLLISTER	6330	41.6	20.0	3.0	36	80.00	4347	10	34	57	42.21	7060	30	114.18	4550	47.8	0.8	5.70	0.0712
8	REYNOLDS	*****	4600	44.5	16.3	3.0	33	90.20	3600	12	51	49	43.15	7200	30	116.45	2841	50.7	0.7	2.90	0.0322
9	TOLLGATE	*****	5950	39.8	22.9	3.0	39	21.00	4600	18	42	58	43.10	7200	29	116.45	2841	50.7	0.7	10.00	0.4762
10	SALMON	*****	4600	44.5	16.1	3.0	30	13.70	3675	12	61	39	43.15	6200	22	116.45	2841	50.7	0.7	3.20	0.2336
11	MACKS	*****	4400	45.2	14.6	2.0	30	12.20	3728	11	55	45	43.15	6200	17	116.45	2841	50.7	0.7	2.60	0.2131
12	RABBIT	*****	5500	41.4	16.0	2.0	25	3.70	4480	13	65	35	43.10	6620	20	116.45	2841	50.7	0.7	2.00	0.5405
13	W MTN	*****	6625	37.5	27.2	2.0	50	0.20	6600	16	32	68	43.05	7000	34	116.50	2841	50.7	0.7	12.40	62.0000
14	TRAPER	OAKLEY	6360	42.5	18.0	2.0	36	53.70	4820	12	33	45	42.10	7795	24	113.59	4600	48.7	0.8	3.70	0.0689
15	COTN	OAKLEY	6500	42.0	26.0	2.5	23	38.00	4700	12	35	39	42.10	8765	40	114.10	4600	48.7	0.8	5.00	0.1316
16	GEORGE	YOST	8570	30.8	25.0	3.0	43	7.80	7000	20	31	61	41.55	9892	30	113.29	4550	44.9	1.0	11.60	1.4872
17	MONT-	PELIER	7500	37.3	25.0	2.0	51	50.90	6210	15	24	72	42.20	9200	35	111.14	5960	42.7	1.0	5.70	0.1120
18	COTNWD	ARROWRK	4900	43.2	21.5	2.0	50	21.40	3090	20	20	60	43.38	7300	25	115.49	3239	49.0	0.7	5.15	0.2407
19	ROBIE	ARROWRK	4960	43.0	25.0	2.0	30	15.80	3080	20	35	50	43.37	6453	30	115.59	3239	49.0	0.7	7.10	0.4494
20	PORTER	GARDEN	4500	41.5	25.0	1.5	46	21.20	3000	20	13	71	43.57	6700	30	116.11	3890	43.6	0.7	7.80	0.3679
21	CABIN	SMITHS	5090	40.0	30.0	3.0	67	0.42	4610	29	5	93	44.21	6000	31	115.47	4865	40.8	0.7	12.80	30.4762
22	L SALM	RIGGINS	5430	40.2	30.0	2.0	48	576.00	1760	15	6	88	45.25	9390	45	116.19	1801	52.9	0.8	20.00	0.0347
23	TYGHEE	HENRY	7500	32.8	25.0	2.0	52	7.30	6800	20	30	66	44.30	8710	30	111.24	6300	37.0	1.1	6.60	0.9041
24	MANN	WEISER	4860	41.7	21.0	1.5	50	56.00	2750	12	31	58	44.24	7640	30	116.54	2120	51.3	0.7	9.80	0.1750
25	JOHNS	PARK	7000	34.2	40.0	2.5	57	5.00	6000	38	6	88	44.45	7940	42	116.45	2935	48.4	0.7	27.80	5.5600
26	WEISER	INDIAN	5300	39.3	28.0	3.0	70	81.90	3250	16	5	90	44.30	7813	40	116.24	4865	40.8	0.7	17.00	0.2076
27	MUD	TAMARACK	4660	38.4	33.0	2.5	65	15.80	3990	26	10	80	45.00	6246	40	116.21	3871	41.2	0.8	16.50	1.0443
28	BOULD	TAMARACK	6500	32.0	36.0	2.5	67	5.90	5000	30	15	80	45.05	8048	42	116.27	3871	41.2	0.8	22.00	3.7288
29	WEISER	TAMARACK	4690	38.3	35.0	2.5	60	36.50	4080	30	16	80	44.57	6000	40	116.23	3871	41.2	0.8	16.20	0.4438
30	S FORK	KNOX	6630	33.9	42.5	2.0	70	92.00	5090	35	11	83	44.39	8696	50	115.42	5375	38.3	0.7	21.40	0.2326
31	E FORK	STIBNITE	7420	31.8	35.0	2.0	50	104.00	5049	30	22	72	44.58	8846	40	115.27	5686	37.9	0.7	17.60	0.1692
32	JOHNSON	YELLOW P	7170	32.7	40.0	2.0	50	213.00	4658	30	13	76	44.58	9169	60	115.30	5686	37.9	0.7	22.20	0.1042
33	E WEISER	COUNCIL	6900	34.1	45.0	3.0	70	2.00	6275	40	10	85	44.46	7180	50	116.16	2936	48.0	0.8	28.80	14.4000
34	MORES	IDA CITY	5000	40.4	25.0	2.0	50	119.00	3550	20	25	65	43.46	8141	50	115.55	3965	44.0	0.7	7.10	0.0597
35	FISH	LOCHSA	4470	39.6	50.0	2.0	50	89.20	1997	40	5	70	46.20	6580	55	115.21	1580	49.7	0.8	38.70	0.4339
36	POTLATCH	BOVILL	3550	42.8	40.0	2.0	70	3.15	2950	35	5	95	46.51	4000	45	116.17	2910	45.0	0.8	21.60	6.8571
37	POTLATCH	ELK R	3900	41.5	45.0	2.0	70	18.20	3000	40	5	90	46.52	5000	55	116.16	2910	45.0	0.8	25.00	1.3736
38	POTLATCH	BOVILL	3500	42.9	40.0	2.0	70	42.50	2800	30	5	90	46.50	5000	50	116.23	2910	45.0	0.8	20.30	0.4776
39	W WEISER	TAMARACK	4940	37.3	30.0	3.0	70	4.00	4200	25	10	85	45.01	6000	40	116.26	3871	41.0	0.8	17.90	4.4750
40	ELK	IDA CITY	6200	36.2	26.5	2.0	50	13.10	4570	20	20	65	43.54	8141	40	115.47	3965	44.0	0.7	13.60	1.0382
41	MORES	IDA CITY	6000	36.9	27.5	2.0	50	37.00	3990	25	20	70	43.50	8141	50	115.47	3965	44.0	0.7	12.30	0.3324
42	PACIFIC	MORAN	8160	29.6	40.0	1.5	48	160.00	6720	20	16	77	43.51	10400	60	110.31	6798	34.4	1.1	22.50	0.1406
43	LA BARGE	R STA	8970	27.7	40.0	2.0	35	6.30	8410	38	21	0	42.30	11418	42	110.40	6115	37.7	1.1	31.50	5.0000

Basic Data for the Original 211 Watersheds Utilized
in the Idaho-Montana-Wyoming Portion of the
Hawley-McCuen Study

Source: R.H. McCuen, Department of Civil Engineering
University of Maryland, College Park, MD

ORIGINAL HAWLEY AND MCCUEN DATA

S D B S	I T E E	L O C	M E A N T M P	E A N T M P	S C I V L D A G E P N	C O O I V L D A G E P N	7.66	9380	M I N I M A V	M B V N R K G L A P	0	43.52	12435	M A X I M A L P G	54	109.18	T E M P E R E M P 5	E L E M E M P 5	P R O D U C T Y	20.50	2.6762
201	WOOD R	KIRWIN	10500	24.5	40.00	2.0	70	7.66	9380	40	15	0	43.52	12435	54	109.18	4313	46.2	1.2	20.50	2.6762
202	NF CRAZY	BUFFALO	7830	35.7	19.50	2.5	20	51.70	5320	15	46	0	44.12	11686	24	106.46	5240	44.8	1.4	4.70	0.0909
203	NF CRAZY	BUFFALO	8010	35.1	19.50	2.5	22	82.70	5190	15	44	0	44.04	10555	24	106.48	5240	44.8	1.4	3.70	0.0447
204	SWEETWATR	STH PASS	8500	34.1	22.00	2.0	28	177.00	7420	12	29	0	42.22	12450	32	108.53	5563	44.4	1.1	5.00	0.0282
205	PRYOR	PRYOR	6000	38.7	18.00	3.5	54	39.60	4440	16	19	0	45.20	7065	20	108.34	3567	47.2	1.2	2.83	0.0715
206	WILLOW	GLASGOW	2536	40.0	13.26	3.0	27	5.00	2500	12	44	0	48.05	2800	14	107.08	2068	40.6	1.4	0.98	0.1960
207	BIG COULEE	LAVINA	4000	43.7	13.00	3.0	19	232.00	3480	12	18	0	46.16	4890	14	108.57	3800	44.4	1.2	0.43	0.0019
208	BOX ELDER	WINNETT	3470	44.5	15.00	3.5	20	684.00	2720	13	22	0	47.01	5585	20	108.09	2887	46.5	1.4	0.45	0.0007
209	BIG MUDDY	*****	2510	37.8	14.00	3.0	30	279.00	2120	12	43	0	48.55	2870	14	105.57	2400	38.2	1.8	0.79	0.0028
210	BOX CR	BILL	4700	45.7	13.07	3.0	50	109.00	4694	12	20	0	43.06	5500	14	105.15	4415	46.7	1.6	0.35	0.0032
211	BOX CR	BILL	5000	44.7	12.54	3.0	50	158.00	4997	12	20	0	43.07	5500	14	105.02	4415	46.7	1.6	0.16	0.0010

Selected Water Resources Abstracts		1. Report No.	2.	3. Accession No. W
Input Transaction Form				
4. Title HYDROLOGIC FLOW DETERMINATION FOR HYDROPOWER FEASIBILITY ANALYSIS			5. Report Date 6. December 1982	
7. Author(s) Filler, J.R. and Warnick, C.C.			8. Performing Organization Report No.	
9. Organization Idaho University, Moscow, Civil Engineering Dept.			10. Project No. A-068-IDA (3)	
12. Sponsoring Organization			11. Contract/Grant No. 14-34-0001-2114	
15. Supplementary Notes Idaho Water and Energy Resources Research Institute Completion Report, Moscow, December 1982. 43 p., 7 fig., 8 tab., 17 ref.			13. Type of Report and Period Covered	
16. Abstract				
<p>This report presents comparisons of different methods of estimating average annual discharge of ungaged streams. The primary purpose of the estimates being for making hydropower feasibility investigations that utilize flow duration curves. The published methods of McDonald (1948), Rosa (1968), Heitz (1980, 1981) and Hawley-McCuen (1982) were used on selected streams in Idaho to make the comparisons. The methods have been developed over a considerable period of time and have used different periods of record but all have used normal annual precipitation estimates as the basis for prediction. Reasonable agreement was obtained in the comparisons but inconsistencies do appear on some drainages.</p> <p>Several additional approaches to improving the estimates and hopefully correcting the inconsistencies have been postulated and techniques for study recommended.</p>				
17a. Descriptors				
Hydrologic Data, Hydropower Feasibility, Parametric Hydrology Streamflow, Runoff Coefficient, Rainfall-Runoff Relationship				
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