## LEAST COST IRRIGATION SYSTEM SPECIFICATIONS FOR VARIOUS CONDITIONS

by

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#### Abstract

SUMMARY: A methodology employing a dynamiclinear programming model was used to develop optimum rehabilitation plans for an irrigation district. The plans developed indicate the total irrigation system cost and configuration for various levels of efficiency and water cost.


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FOR VARIOUS CONDITIONS
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## INTRODUCTION

Multi-objective irrigation system planning is a complex process when considering both water distribution and application systems. Planners must consider not only the objectives of irrigators but also other portions of the public that might be affected by a new irrigation system or the rehabilitation of an existing system. The system plan selected for implementation should effectively integrate distribution and application system components to provide the "best" system as specified by the various objectives.

Irrigation system plans are also subject to various constraining conditions. These conditions include physical, social and economic factors, and are necessary for specifying the relationships of individual components with one another. They are also necessary establishing boundary conditions around the entire system.

A methodology has been developed to enable the rapid evaluation of many different irrigation system alternatives (Busch, 1974, 1975). The methodology uses a two-stage dynamic-linear programming model to select optimal system configurations. Input to the model consists of economic and physical data for each component under consideration. Several alternative components may be specified for each portion of the system, and the model is used to select the optimal (least cost) combination of components that is subject to all specified constraints.

To test the model developed, an old irrigation district in eastern Idaho was selected, and least cost rehabilitation plans were developed. It is the purpose of this paper to describe the application of the model and present the optimum rehabilitation plans obtained.

## STUDY AREA

The area selected for application of the optimization procedure is that served by the North Rigby Irrigation and Canal Company. It is located in Jefferson County, Idaho; and encompasses approximately 990 irrigated acres. The area served, shown in Figure 1, is less than one mile wide and approximately four miles long.

Irrigation water is supplied from the Great Feeder Canal through a distribution system that is relatively unchanged from when it was constructed during the 1880's. Approximately half of the diversion and drop structures are made of concrete with the other half being made of wood. No water measuring devices are installed in the system. Maintenance work is done by the water users using


Figure 1. North Rigby Irrigation District.
farm equipment, and periodically a small bulldozer is used to clean and reshape sections of the main canal.

Soils present in the irrigation district are shown in Figure 1. All soils have medium to high intake rates and are underlain by sands and gravels. As a result, field irrigation efficiencies are quite low in the 20-50 percent range (Galinato, 1974). Canal sections often penetrate shallow soils and have gravel or sand bottoms. Brockway and de Sonneville (1973) report an average seepage rate $3.5 \mathrm{ft} / \mathrm{day}$ from canals in the area. High intake rates combined with a rather antiquated system make for an overall irrigation efficiency of less than 20 percent for the district.

## PROCEDURE

Components within an irrigation system may be grouped into two main categories. First are those used to apply water to the land, application systems. The second, distribution systems, are used to convey and distribute water to the application systems. Physical and economic inputs are used to compute the cost and efficiency of each component functioning within a given irrigation district.

## App1ication Systems

Input to the model requires annual costs of application systems on a per-acre basis.

$$
\begin{equation*}
\text { Annual Cost }=\mathrm{cN} \tag{1}
\end{equation*}
$$

where

$$
\begin{aligned}
& \mathrm{c}=\text { annual cost per acre } \\
& \mathrm{N}=\text { number of acres supplied }
\end{aligned}
$$

Also required is the application efficiency (percentage of water applied retained in the root zone) of each system under consideration. As the cost and application efficiency of application systems are dependent on several factors such as soil, slope, farm size, field size and crops grown, it is necessary to break the irrigation district into several units shown in Figure 2. The unit boundaries are influenced by soils, field size and crops grown, and coincide quite closely with the soils boundaries shown in Figure 1.


Figure 2. Units within North Rigby Irrigation District.

Alternative application systems considered include both gravity and sprinkler systems listed in Tables 1 and 2. Annual costs for the different sizes of systems listed were obtained for every crop grown on each of the different soils. The costs include the total annual costs of applying water and conveying water from the point of diversion to the point or points of application.

## Distribution Systems

The annual cost of each distribution system component is specified as a function of the maximum flow rate conveyed, controlled or pumped by that component. Also, for any component there is a minimum cost associated with construction, operation and maintenance, the fixed specified cost. Therefore, the total annual cost of the component is:

$$
\begin{equation*}
\text { Annual } \operatorname{Cos} t=c Q+d \tag{2}
\end{equation*}
$$

where

$$
\begin{aligned}
& c=\text { annual cost per unit volume flow rate } \\
& Q=\text { maximum volume flow rate } \\
& d=\text { annual fixed specified cost }
\end{aligned}
$$

The conveyance efficiency of each component is also computed.
Four main distribution system alternatives were considered for the irrigation district; gravity, pressure-pipe, wells with high head pumps and wells with low head pumps. The gravity system is designed to supply water from the Great Feeder Canal and deliver it to farms at zero pressure. The pressure pipe system is also supplied from the Great Feeder Canal by a pumping plant with sufficient head to supply a sprinkler system without need of a booster pump. Wells with low head pumps are designed to deliver water at zero pressure, and those with high head pumps to deliver water at sprinkler operating pressure. Maximum static lift for all pumps is considered to be 110 feet or less based upon the findings of Brockway and de Sonneville (1973).

Three types of conveyance, unlined canal, lined canal and gravity pipeline, were considered for each section of the gravity distribution system. Sections A through H of the unlined canal route are shown in Figure 3. Junction locations are the same for all three types of gravity conveyance, and the pressure head is near zero at each junction. Therefore, combinations of the three types may be specified to make up a gravity distribution system. The number of possible combinations considering each of the three types for each of N sections is 3 N .

A dynamic programming procedure (Busch, 1974, 1975) was used to eliminate more costly, less efficient dustribution system combinations. Application of the procedure results in a reduction of gravity system component combinations to 54 for the irrigation district. The dynamic programming procedure was not applied to the other distribution system alternatives. There are no different types of components for any section of the pressure-pipe system, and the same is true for the wel1-pump combinations.

Table 1. Surface application systems considered for the North Rigby Irrigation District

| System Type | Symbol | General Description |
| :--- | :--- | :--- |
| Unimproved gravity | UG | The system consists of poorly <br> maintained earthen ditches with <br> earthen and wooden structures <br> and portable canvas dams used <br> for water control. Maximum <br> allowable length of irrigation <br> run is l30 feet. |
| Improved gravity | IG | The system consists of well <br> maintainedearthen ditches with <br> concrete and metal structures |
|  |  | used for water control. Maxi- <br> mum allowable length or irriga- <br> tion run is 650 feet. A cross <br> ditch is specified if the irri- <br> gation run is in excess of the <br> 650 foot length. |

Table 2. Sprink1er app1ication systems considered for the North Rigby Irrigation District

| System Type | Symbol | General Description |
| :--- | :--- | :--- |
| Hand-line sprinkler | HS | The layout of the system con- <br> sists of hand-carried laterals <br> supplied by a permanent or <br> semi-permanent mainline. |
| Hand-1ine sprinkler <br> with pump | HSP | Same as hand-1ine sprinkler but <br> equipped with a pump to supply <br> pressure. |
| Side-roll sprinkler | RS | The layout of the system con- <br> sists of mechanicaliy moved <br> laterals supplied by a perma- <br> nent or semi-permanent mainline. |
| Side-roll sprinkler | RSP | Same as side-roll system, but <br> with pump |



Figure 3. Un1ined canal route and sections for North Rigby Irrigation District.

Cost-discharge relationships for all distribution system components are estimated quite well by the 1inear Equation 2. The correlation coefficients relating estimated and computed values are greater than 0.90 for all components considered.

## Linear-Programming Problem Formulation

The linear cost functions for alternative application systems and distribution system components, and operation and maintenance costs are combined in a linear-programming model. Model constraints assure that all crops within the district receive an adequate supply of water to meet maximum consumptive use requirements. The mode1 is revised for each different distribution system combination alternative.

Least cost system specifications are obtained as optimal solutions from the linear-programming model. The total system cost and all individual component costs are obtained. In addition, water flow rates through each distribution system component and acreages covered by each type of application system are specified. Post optimal analyses may be performed to vary costs and constraining parameters and to determine the sensitivity of any optimal solution to individual parameter changes.

## RESULTS

Optimum rehabilitation plans for the North Rigby Irrigation District were obtained to meet different specified conditions. Various levels of overall system efficiency (percentage of water diverted to consumptive use) were specified, and the price of water entering the system was allowed to vary over a specified range. The effects of these parameter changes were computed separately, and the results will be described separately.

## Effects of Changes in System Efficiency

The results of optimal solutions for different distribution system alternatives are summarized in Tables 3,4 and 5. The range of efficiencies shown for each alternative is the attainable range for optimal solutions.

For the gravity distribution system, solutions were obtained for a range in efficiency from the present 17.1 percent to 70 percent as shown in Table 3. Corresponding system costs vary from $\$ 67,523$ to $\$ 93,179$. The maximum flow rates required are given as are the annual losses to deep percolation and surface runoff. These losses can be used to determine the system contribution to subsurface and surface drainage in the area.

Component configuration for the distribution and application systems is specified for each efficiency level. Up to and including a specified efficiency of 60 percent, the greatest change in components occurs with the application systems changing from unimproved gravity to sprinkler. Very little change is present in the distribution system at specified efficiencies less than 70 percent. These results indicate that for a gravity distribution system it would be more economical for water users to improve overall system efficiency by using sprinkler application systems rather than by undertaking a major rehabilitation of the distribution system.

The summary for 10 head we 11 supply in Table 4 shows least cost solutions for efficiencies from 38.4 to 70 percent. At efficiencies less than 38.4 percent, the cost of pumping water is greater than the cost of improving application systems. The small variation in total annual cost over the entire efficiency range is due to the fact that pumping costs decrease as application systems are upgraded and efficiency increases.

Efficiencies of 70 percent only are shown for the pressure pipeline and high head well supplies as these alternatives supply only sprinkler systems designed to operate at 70 percent efficiency. A comparison of the total costs shows that the cost of wells with high head pumps supplying sprinkler systems at 70 percent efficiency is less than any other alternative except for the gravity distribution system operating at 17.1 percent.

It should be emphasized that costs associated with different systems are somewhat of a different nature. For example, much of the annual cost associated with an unimproved gravity type distribution and application system is paid out for manual labor, management, and machine hire furnished by farmer irrigators. However, much of the annual cost associated with a side-roll sprinkler system supplied by a well with a high head pump is money required to repay a high initial capital investment.

## Effects of Changes in Water Costs

Charges for water are often assessed for surface water delivered to an irrigation district by a feeder canal. The basis for charges

Table 3. Total annual system costs for varying efficiencies for a gravity distribution system

| System efficiency (\%) | 17.1 | 20 | 30 | 40 | 50 | 60 | 70 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Total annual cost (\$) | 67,523 | 68,931 | 73,329 | 76,826 | 79,586 | 81,851 | 93,179 |
| Max. flow rate (cfs) | 56.9 | 48.7 | 32.4 | 24.3 | 19.5 | 16.2 | 13.9 |
| Volume to DP (AF) | 3326 | 2563 | 1409 | 944 | 844 | 721 | 399 |
| Volume to SR (AF) | 3554 | 3048 | 1816 | 1097 | 445 | 65 | 0 |
| Distribution system Section A | UC ${ }^{\text {a }}$ | UC | UC | UC | UC | UC | LC |
| B | UC | UC | UC | UC | UC | UC | GP |
| C | UC | UC | UC | UC | UC | UC | GP |
| D | UC | LC | LC | LC | LC | LC | GP |
| E | UC | UC | UC | GP | GP | GP | GP |
| F | UC | UC | UC | UC | UC | UC | GP |
| G | UC | UC | UC | UC | UC | UC | LC |
| H | UC | UC | UC | UC | UC | UC | GP |
| Application system <br> Unit I | UG ${ }^{\text {b }}$ | UG | UG | RSP | RSP | RSP | RSP |
| II | UG | RSP | RSP | RSP | RSP | RSP | RSP |
| III | UG | UG | HSP | HSP | HSP | HSP | HSP |
| IV | UG | IG | $\begin{gathered} \operatorname{HSP}(30 \% \\ \text { IG }(70 \% \end{gathered}$ | ) HSP | HSP | HSP | HSP |
| V | UG | UG | UG | $\begin{gathered} \text { RSP ( } 17 \% \text { ) } \\ \text { UG }(83 \%) \end{gathered}$ | $\begin{gathered} \operatorname{RSP}(79 \%) \\ \text { UG (11\%) } \end{gathered}$ | ) RSP | RSP |
| VI | UG | $\begin{gathered} \text { RSP ( } 15 \% \text { ) } \\ \text { UG ( } 85 \% \text { ) } \end{gathered}$ | RSP | RSP | RSP | RSP | RSP |
| VII | UG | UG | UG | UG | UG R | $\begin{gathered} \text { RSP ( } 70 \% \text { ) } \\ \text { UG ( } 30 \% \text { ) } \end{gathered}$ | RSP |

[^0]Table 4. Total annual system costs for varying efficiencies for a low head well supply

| System efficiency (\%) | 38.4 | 40 | 50 | 60 | 70 |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Total annual cost (\$) | 86,738 | 86,785 | 87,007 | 87,165 | 87,739 |
| Max. flow rate (cfs) | 25.3 | 24.3 | 19.5 | 16.2 | 13.9 |
| Volume to DP (AF) | 523 | 520 | 505 | 501 | 399 |
| Volume to SR (AF) | 1618 | 1467 | 739 | 256 | 0 |
| Application system Unit I | UG ${ }^{\text {a }}$ | UG | UG | $\begin{gathered} \operatorname{RSP}(89 \%), \\ \operatorname{UG}(11 \%) \end{gathered}$ | RSP |
| II | RSP | RSP | RSP | RSP | RSP |
| III | HSP | HSP | HSP | HSP | HSP |
| IV | HSP | HSP | HSP | HSP | HSP |
| V | UG | $\begin{gathered} \operatorname{RSP}(14 \%), \\ \text { UG }(86 \%) \end{gathered}$ | $\begin{gathered} \text { RSP (83\%), } \\ \text { UG(17\%) } \end{gathered}$ | RSP | RSP |
| VI | RSP | RSP | RSP | RSP | RSP |
| VII | UG | UG | UG | UG | RSP |

[^1]Table 5. Total annual system costs for varying efficiencies for high pressure pipeline and high head well supplies

|  | Pressure pipeline | Wells with high head pumps |
| :---: | :---: | :---: |
| System efficiency (\%) | 70 | 70 |
| Total annual cost (\$) | 75,121 | 68,769 |
| Max. flow rate (cfs) | 13.9 | 13.9 |
| Volume to DP (AF) | 399 | 399 |
| Volume to SR (AF) | 0 | 0 |
| Application system Unit I | $\mathrm{RS}^{\text {a }}$ | RS |
| II | RS | RS |
| III | HS | HS |
| IV | HS | HS |
| V | RS | RS |
| VI | RS | RS |
| VII | RS | RS |

a Application system symbols in Table 2
can vary. A common basis is cost per unit volume, usually
dollars per acre-foot.
The charge for surface water entering the North Rigby Irrigation District was allowed to vary from $\$ 0$ per acre-foot to $\$ 12$ per acrefoot. These charges were considered for both gravity and pressure distribution systems but not for wells as charges are seldom assessed against pumped groundwater. Results related to the various water costs are summarized in Tables 6 and 7 .

The data in Table 6 indicate that system configuration does not change for water costs of $\$ 8$ per acre-foot or greater. Also, application system components are the first to change with increas ing water cost as they were with increasing specified system efficiency. This fact indicates that the amount of water saved versus cost is generally greater for application system components than for distribution system components.

| Water cost (\$/AF) | 0 | 2 | 4 | 6 | 8 | 10 | 12 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| System cost (\$) | 67,523 | 84,318 | 92,718 | 98,394 | 103,537 | 108,509 | 113,480 |
| System efficiency (\%) | 17.1 | 26.0 | 54.8 | 62.5 | 67.0 | 67.0 | 67.0 |
| Max. flow rate (cfs) | 56.9 | 37.0 | 17.8 | 15.6 | 14.5 | 14.5 | 14.5 |
| Volume to DP (AF) | 3326 | 1599 | 809 | 685 | 505 | 505 | 505 |
| Volume to SR (AF) | 3554 | 2294 | 219 | 0 | 0 | 0 | 0 |
| Distribution system Section A | Uc ${ }^{\text {a }}$ | UC | UC | UC | LC | LC | LC |
| B | UC | UC | UC | UC | LC | LC | LC |
| C | UC | UC | UC | UC | UC | UC | UC |
| D | UC | LC | LC | LC | LC | LC | LC |
| E | UC | UC | GP | GP | GP | GP | GP |
| F | UC | UC | UC | UC | UC | UC | UC |
| G | UC | UC | UC | UC | LC | LC | LC |
| H | UC | UC | UC | UC | GP | GP | GP |
| Application system Unit I | UG ${ }^{\text {b }}$ | UG | RSP | RSP | RSP | RSP | RSP |
| II | UG | RSP | RSP | RSP | RSP | RSP | RSP |
| III | UG | UG | HSP | HSP | HSP | HSP | HSP |
| IV | UG | IG | HSP | HSP | HSP | HSP | HSP |
| V | UG | UG | RSP | RSP | RSP | RSP | RSP |
| VI | UG | RSP | RSP | RSP | RSP | RSP | RSP |
| VII | UG | UG | UG | RSP | RSP | RSP | RSP |

[^2]Table 7. Total annual system costs for varying water costs for a high pressure pipeline system

| Water cost (\$/AF) | Overall annual system cost ${ }^{\mathrm{a}}$ <br> Pressure pipeline $(\$)$ |
| :---: | :---: |
| 0 | 75,121 |
| 2 | 79,881 |
| 4 | 84,641 |
| 6 | 89,401 |
| 10 | 94,161 |
| 12 | 98,921 |

${ }^{a}$ System configuration is identical to those in Table 5

Based upon the results obtained and considering only the total annual cost, the most economical way to increase the overall irrigation efficiency of the given district is to abandon all present systems and to install wells from which water could be pumped to supply sprinkler systems. System costs increase quite drastically if a charge for water entering the system from a surface source is considered. However, an increasing water charge would also force a marked increase in system efficiency. Charging for water would add a cost requiring money to be spent outside the district.

Various other plans and incentives for rehabilitation could easily be considered as changes in the linear-programming model require minimal modification of the modeled problem. Specific planning needs would dictate which relationships and constraining conditions would be most meaningful for the district considered.

Results obtained from the model can be used to develop specific designs for system components. The cost of the resultant design for the entire system would be nearly the same as the optimum cost obtained from the modeled problem.

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[^0]:    $\mathrm{a}_{\mathrm{UC}}=$ unlined canal, $\mathrm{LC}=$ lined canal, and $\mathrm{GP}=$ gravity pipeline
    ${ }^{\mathrm{b}}$ Application system symbols in Tables 1 and 2

[^1]:    ${ }^{\text {a Application system symbols in Tables } 1 \text { and } 2}$

[^2]:    $\mathrm{a}_{\mathrm{UC}}=$ unlined canal, $\mathrm{LC}=$ lined cana1, $\mathrm{GP}=$ gravity pipeline
    bapplication system symbols in Tables 1 and 2

